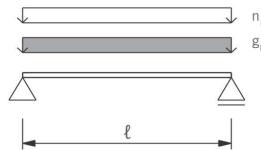


## Single span beam - vibration



Minimum panel thickness of various fire resistance classes.

<b>Dead load</b>	<b>Imposed load</b>	<b>Span of single - span beam</b>									
		$g_k^*$	$n_k$	3.0m	3.5m	4.0m	4.5m	5.0m	5.5m	6.0m	6.5m
1,0	1,0	80 L3	80 L3	90 L3	120 L3	120 L3	140 L5	160 L5/2	180 L5	200 L5	220 L7/2
	2,0		90 L3	100 L3	120 L3	120 L3	140 L5	160 L5/2	180 L5		
	2,8		80 L3	90 L3	120 L3	120 L3	140 L5	160 L5/2	180 L5		
	3,5	80 L3	100 L3	120 L3	120 L3	140 L5	160 L5/2	180 L5	200 L5	220 L7/2	240 L7/2
	4,0	90 L3	100 L3	120 L3	120 L3	140 L5	160 L5/2	180 L5	200 L5		
	5,0	90 L3	120 L3	120 L3	120 L3	140 L5	160 L5/2	180 L5	200 L5	220 L7/2	240 L7/2
1,5	1,0	80 L3	90 L3	100 L3	120 L3	120 L3	140 L5	160 L5/2	180 L5	200 L5	220 L7/2
	2,0	80 L3	90 L3	120 L3	120 L3	120 L3	140 L5	160 L5/2	180 L5		
	2,8		100 L3	120 L3	120 L3	140 L5	160 L5/2	180 L5	200 L5	220 L7/2	240 L7/2
	3,5		90 L3	120 L3	120 L3	140 L5	160 L5/2	180 L5	200 L5		
	4,0	90 L3	120 L3	120 L3	140 L5	160 L5/2	180 L5	200 L5	220 L7/2	240 L7/2	260 L7/2
	5,0	90 L3	120 L3	120 L3	140 L5	160 L5/2	180 L5	200 L5	220 L7/2		
2,0	1,0	80 L3	100 L3	120 L3	120 L3	120 L3	140 L5	160 L5/2	200 L5	220 L7/2	240 L7/2
	2,0	90 L3		120 L3	120 L3	120 L3	140 L5	160 L5/2	180 L5		
	2,8	90 L3		120 L3	120 L3	120 L3	140 L5	160 L5/2	180 L5		
	3,5	90 L3	120 L3	120 L3	120 L3	140 L5	160 L5/2	180 L5	200 L5	220 L7/2	240 L7/2
	4,0	100 L3	120 L3	120 L3	140 L5	160 L5/2	180 L5	200 L5	220 L7/2		
	5,0	100 L3	120 L3	120 L3	140 L5	160 L5/2	180 L5	200 L5	220 L7/2	240 L7/2	260 L7/2
2,5	1,0	90 L3	100 L3	120 L3	120 L3	140 L5	160 L5/2	160 L5/2	200 L5	220 L7/2	240 L7/2
	2,0	90 L3	120 L3	120 L3	120 L3			180 L5	200 L5		
	2,8		120 L3	120 L3	120 L3			200 L5	220 L7/2		
	3,5		140 L5	160 L5/2	160 L5/2			200 L5	220 L7/2		
	4,0	100 L3	120 L3	120 L3	140 L5	160 L5/2	160 L5/2	180 L5	200 L5	240 L7/2	260 L7/2
	5,0	100 L3	120 L3	120 L3	140 L5	160 L5/2	180 L5	200 L5	220 L7/2		
3,0	1,0	90 L3	90 L3	120 L3	120 L3	140 L5	160 L5/2	180 L5	220 L7/2	240 L7/2	260 L7/2
	2,0	90 L3	90 L3	120 L3	120 L3	140 L5	160 L5/2	180 L5			
	2,8	100 L3	120 L3	120 L3	140 L5	160 L5/2	180 L5	200 L5			
	3,5		120 L3	120 L3	140 L5	160 L5/2	180 L5	200 L5	240 L7/2	260 L7/2	280 L7/2
	4,0		120 L3	120 L3	140 L5	160 L5/2	180 L5	200 L5			
	5,0	100 L3	120 L3	120 L3	140 L5	160 L5/2	180 L5	200 L5	240 L7/2	260 L7/2	280 L7/2

**Service class 1, Imposed load category A ( $\psi_0 = 0.7$ ;  $\psi_1 = 0.5$ ;  $\psi_2 = 0.3$  ),  $k_{mod} = 0.8$**

\*In the table the CLT self weight is already taken into account.

### Serviceability:

a) Quasi-constant design situation: zul  $w_{fin}=250$

b) Infrequent design situation: zul  $w_{q,inst} = 300$ ;

zul  $w_{fin} - w_{g,inst} = 200$ ;

$k_{def} = 0.6$

c)Vibration according to EN 19951-1-1 and Kreuzinger & Mohr

( $f_1 > 8$  Hz or  $f_1 > 5$  Hz with  $a = 0.4$  m/s<sup>2</sup>,  $V < V_{grenz}$ ,  $W_{EF} < 1$  mm)

D = 2%, 5 cm cement screed, b = 1.2 \* l

**This table specifies the required thicknesses for the normal design situation (R0).**

**The table is only for preliminary estimate purpose and is not a substitute**

**for a structural analysis.**

### Loading - bearing capacity:

a) verification of bending stresses;

b) Verification of shearing stresses;

$k_{mod}=0.8$

### Fire resistance

$v_{1,j} = 0.63$  mm/min

$v_{2,j} = 0.86$  mm/min

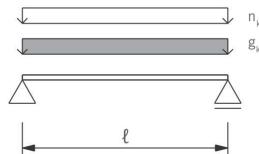
R0

R30

R60

R90

## Single span beam - deformation



Minimum panel thickness of various fire resistance classes.

Dead load	Imposed load	Span of single - span beam										
		$g_k^*$	$n_k$	3.0m	3.5m	4.0m	4.5m	5.0m	5.5m	6.0m	6.5m	7.0m
1,0	1,0			80 L3	90 L3		120 L3		140 L5		160 L5/2	180 L5
	2,0			80 L3	90 L3	100 L3	120 L3	120 L3			160 L5/2	200 L5
	2,8			80 L3	90 L3	120 L3	120 L3	140 L5			180 L5	
	3,5			80 L3	100 L3		120 L3					
	4,0			80 L3	100 L3		140 L5					
	5,0			90 L3	120 L3	120 L3		160 L5/2		200 L5	220 L7/2	
1,5	1,0			80 L3	90 L3	100 L3	120 L3		140 L5		160 L5/2	180 L5
	2,0			80 L3	80 L3	120 L3	120 L3	140 L5				200 L5
	2,8			80 L3	100 L3		120 L3					
	3,5			90 L3			120 L3					
	4,0			90 L3			140 L5					
	5,0			90 L3	120 L3			160 L5/2		200 L5	220 L7/2	
2,0	1,0			80 L3	100 L3	120 L3	120 L3	140 L5		180 L5	200 L5	
	2,0			90 L3					160 L5/2			
	2,8			90 L3	120 L3	120 L3						
	3,5			90 L3	120 L3	120 L3	140 L5	160 L5/2	180 L5			
	4,0			90 L3						200 L5	220 L7/2	
	5,0			100 L3	120 L3				180 L5			
2,5	1,0			90 L3	100 L3	120 L3			160 L5/2	180 L5		
	2,0			90 L3			120 L3			200 L5		
	2,8			90 L3	120 L3	120 L3	140 L5	160 L5/2	180 L5		220 L7/2	
	3,5			100 L3						200 L5		
	4,0			100 L3	120 L3		140 L5	160 L5/2		220 L7/2		
	5,0			100 L3	120 L3				200 L5			
3,0	1,0			90 L3		120 L3	140 L5		160 L5/2	180 L5		
	2,0			90 L3	120 L3					200 L5		
	2,8			100 L3		140 L5		160 L5/2			220 L7/2	
	3,5			100 L3	120 L3		160 L5/2					
	4,0			100 L3						220 L7/2		
	5,0			100 L3	120 L3				180 L5			

**Service class 1, Imposed load category A ( $\psi_0 = 0.7$ ;  $\psi_1 = 0.5$ ;  $\psi_2 = 0.3$ ),  $k_{mod} = 0.8$** 

\*In the table the CLT self weight is already taken into account.

**Serviceability:**a) Quasi-constant design situation: zul  $w_{fin} = 250$ b) Infrequent design situation: zul  $w_{q,inst} = 300$ ;zul  $w_{fin} - w_{g,inst} = 200$ ; $k_{def} = 0.6$ **Loading - bearing capacity:**

a) verification of bending stresses;

b) Verification of shearing stresses

 $k_{mod}=0.8$ **Fire resistance**

HFA 2011

 $v_1 = 0.65 \text{ mm/min}$ 

R0

R30

R60

R90

**This table specifies the required thicknesses for the normal design situation (R0).****The table is only for preliminary estimate purpose and is not a substitute for a structural analysis.**